



Geotechnical centrifuge model tests for explosion cratering and propagation laws of blast wave in sand*

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Abstract: This paper presents the explosion cratering effects and their propagation laws of blast waves in dry standard sands using a 450 g-t geotechnical centrifuge apparatus. Ten centrifuge model tests were completed with various ranges of explosive mass, burial depth and centrifuge accelerations. Eleven accelerometers were installed to record the acceleration response in sand. The dimensions of the explosion craters were measured after the tests. The results demonstrated that the relationship between the dimensionless parameters of cratering efficiency and gravity scaled yield is a power regression function. Three specific function equations were obtained. The results are in general agreement with those obtained by other studies. A scaling law based on the combination of the π terms was used to fit the results of the ten model tests with a correlation coefficient of 0.931. The relationship can be conveniently used to predict the cratering effects in sand. The results also showed that the peak acceleration is a power increasing function of the acceleration level. An empirical exponent relation between the proportional peak acceleration and distance is proposed. The propagation velocity of blast waves is found to be ranged between 200 and 714 m/s.

Key words: Centrifuge model tests, Explosion, Craters, Blast waves, Sand

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1 Introduction

The geotechnical centrifuge model test, which has been widely used in the past 80 years (Craig, 1989; Taylor, 1995), especially in Russia (the former Soviet Union), UK and USA, provides inertial forces by rotating the model at a high acceleration level, thus producing a stress field in a small-scale model corresponding to that in a prototype. For a centrifuge dynamic test, a small model at Ng (N times acceleration of earth's gravity) with energy E is similar to a large event at g with energy equal to N^3E (Schmidt and Holsapple, 1980). It means an experiment with

1 g of explosives at 100g can be used to simulate an experiment with 1 t in the field. Pokrovsky and Fyodorov (1965; 1969) studied the effects of buried explosives used in the construction industry by centrifuge tests. In the 1930s, a dry sand and a moist clay were used. The results of the apparent crater were compared to those in the field. Holsapple and Schmidt (1979; 1987), Piekutowski (1980), Housen *et al.* (1983), Schmidt and Housen (1987), and Housen and Holsapple (2003; 2011) introduced "point-source" approximations for the impactor and explosive. A coupling parameter, C , was proposed to describe the impactor or explosive. The volume, shape of crater, and cratering time can be derived by this coefficient by dimensional analysis. Lin *et al.* (1994) conducted model tests with small charges, various ranges of burial depths and centrifuge acceleration levels in a lunar soil. The results indicated that there was an

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optimum depth of burial of eight charge diameters for obtaining the maximum apparent crater volumes. Simpson *et al.* (2005) investigated explosive cratering on earth-filled embankment dams by geotechnical centrifuge model tests. The explosives were placed and detonated on the surface of the dam crest. The reference values of explosive charges that resulted in dam break were evaluated based on experimental results. Ma *et al.* (2010) carried out centrifuge model tests for responses of shallow-buried circular structures under surface blasting. These tests were completed at a single acceleration level by using the Tsinghua University 50 *g*-t geotechnical centrifuge. Compared with the field tests, centrifuge model tests are advantageous in their high efficiency, security, repeatability, and low cost.

A centrifuge modeling test system for explosion was developed on the China Institute of Water Resources and Hydropower Research (IWHR) geotechnical centrifuge apparatus. Ten model tests for buried explosion in standard sand with varying explosive charges, depths of burst and acceleration levels were completed. Both explosion cratering and propagation laws of blast wave in sand were investigated. Finally, the results and main findings of these tests were summarized.

2 Experimental

2.1 Equipment and materials

The IWHR geotechnical centrifuge apparatus has a design capacity of 450 *g*-t, a maximum model acceleration of 200*g* in dynamic tests and a maximum payload mass of 1500 kg.

The model container is a thick-walled round steel bucket with internal dimensions of $\Phi 700$ mm \times 700 mm as shown in Fig. 1. A JMK F3.6 mm camera was used to monitor the sand motions.

The standard sand used in these tests was produced in Pingtan County, Fujian Province of China. It is processed in accordance with the industry standards of China, JTJ051-93 and JTJ059-95. Table 1 lists the grain size distribution of the sand. The water content of the tested sand is close to 0.

The No. 8 detonator caps were used as explosive sources. One cap is equal to 1 g of hexogen explosives (RDX).

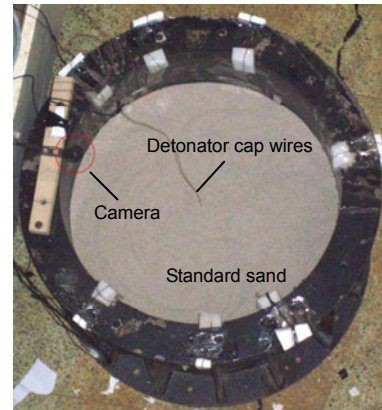


Fig. 1 Test model with explosive and camera

Table 1 Grain size distribution of the sand

Grain size (mm)	Finer percentage (%)
1	100
0.5	50.5
0.25	13
0.1	0.5

2.2 Experimental arrangement

Ten centrifuge model tests were completed with various ranges of explosive mass, burial depth and centrifuge accelerations as listed in Table 2. Test GE is completed on ground, and Test CE-*x* (*x*=1,2,...,9) are completed using centrifuge apparatus. The main features of the ten tests involving two groups of explosive charges of 1 and 3 g. The explosives were buried 100 and 150 mm deep from the surfaces of the sand fills that had total heights of 450 and 500 mm, respectively. The locations of the explosives, as well as those of the surrounding accelerators, in reference to the container kept unchanged, are shown in Fig. 2a. These two cases required 273 and 298 kg sand, respectively. Consequently, there are two values of sand density, namely, 1.58×10^3 and 1.46×10^3 kg/m³. The explosives were detonated at the corresponding acceleration levels.

Eleven accelerometers were installed in the sand to monitor the blast waves. The locations of the accelerometers are shown in Fig. 2 (Fan *et al.*, 2012), and the detailed coordinates of the accelerometers are shown in Table 3.

Table 2 Main features of the tests

Test No.	ρ (kg/m ³)	W (g)	Depth of burial (mm)	Acceleration level ($\times g$)
GE	1.58×10^3	1	100	1
CE-1	1.58×10^3	1	100	40
CE-2	1.58×10^3	1	100	70
CE-3	1.58×10^3	1	100	100
CE-4	1.58×10^3	3	100	40
CE-5	1.58×10^3	3	100	70
CE-6	1.58×10^3	3	100	100
CE-7	1.46×10^3	3	150	40
CE-8	1.46×10^3	3	150	70
CE-9	1.46×10^3	3	150	100

W : charge mass; ρ : sand density

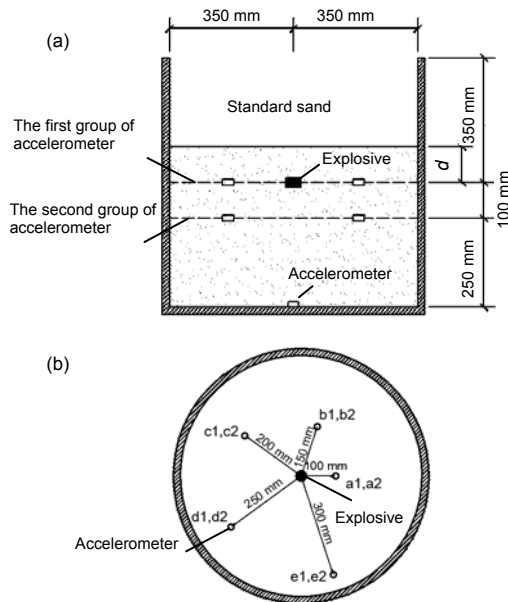


Fig. 2 Layout of experiment (Fan et al., 2012)

(a) Profile of the test model; (b) Arrangement of accelerometers in the first and the second groups

Table 3 Coordinates of the accelerometers

No.	Distance from the explosive		
	Horizontal (mm)	Vertical (mm)	Straight line (mm)
a1	100	0	100.0
b1	150	0	150.0
c1	200	0	200.0
d1	250	0	250.0
e1	300	0	300.0
a2	100	100	141.4
b2	150	100	180.3
c2	200	100	223.6
d2	250	100	269.3
e2	300	100	316.2
bb*	0	350	350.0

* No. bb was placed at the bottom of the bucket

When explosives were detonated at a scheduled acceleration level, a crater would be created in the standard sand. At the same time, the accelerometers recorded the acceleration response. The rotational speed of the centrifuge was reduced until it stopped. The craters were measured using a special vernier caliper. The measurement technique was achieved by placement of the vernier endpoint of contact with the sand surface. The accuracy of crater profile measurements along a given radial at 10 or 20 mm interval is within ± 0.02 mm in the vertical direction.

3 Results and discussion

3.1 Crater dimensions

Fig. 3 shows the pictures of sand motion and a typical crater, which were taken by camera. Fig. 3a was recorded at the moment of sand splash, indicating that the sand was thrown up when the explosion occurred. Fig. 3b shows a typical crater. This crater was formed after the splashed sand fell back. It is known as an apparent crater that was measured along the dotted line in Fig. 3b.

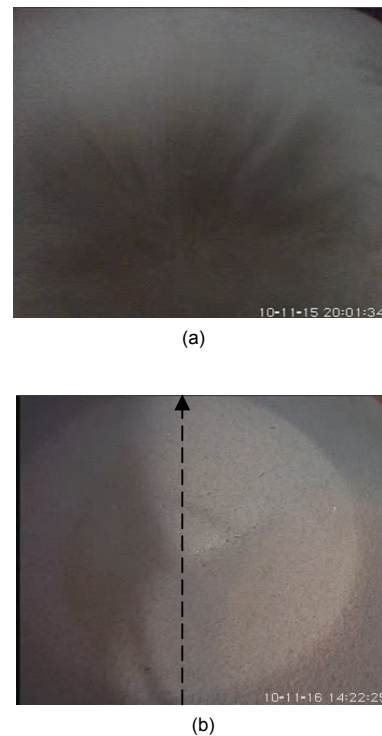


Fig. 3 Pictures of sand motion (a) and a typical crater (b)

The shapes of the ten apparent craters, depicted as half profiles in a representative radial direction, are shown in Fig. 4, respectively. Fig. 4a shows the four test results with 1 g of charge at 100 mm depth of burst when the acceleration level was increased from g to 100g. Fig. 4b shows three test results with 3 g of charge at 100 mm depth of burst when the acceleration level was increased from 40g to 100g. Fig. 4c shows three test results with 3 g of charge at 150 mm depth of burst when the acceleration was increased from 40g to 100g. We can find that when the acceleration level is increased, the craters are less deep.

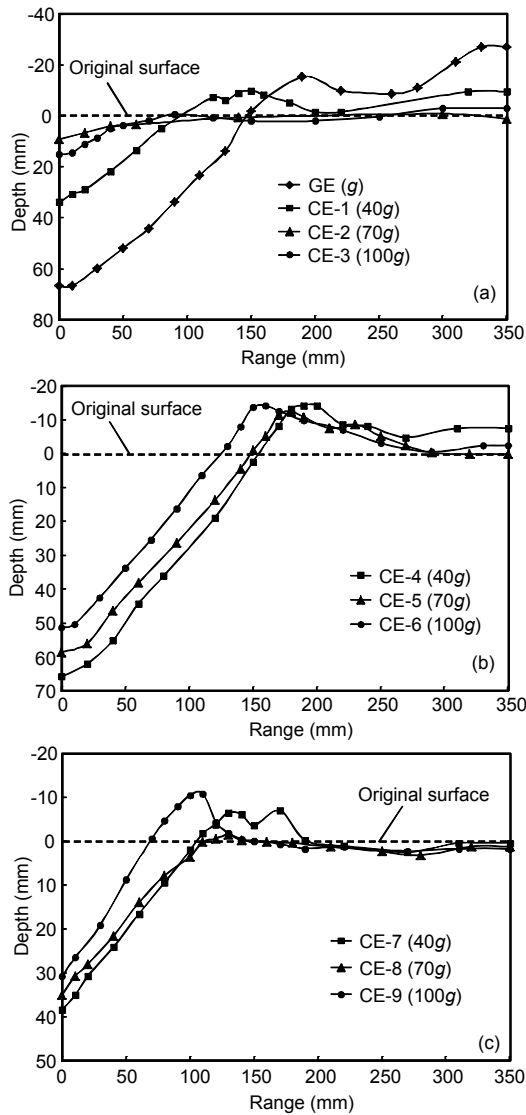


Fig. 4 Apparent crater half profiles in 1 g of charge, 100 mm depth of burst (a), 3 g of charge, 100 mm depth of burst (b), and 3 g of charge, 150 mm depth of burst (c)

In Fig. 4a, the craters of tests CE-2 and CE-3 are abnormally small because its energy involved in 1 g of charge cannot create an obvious crater at 70g and 100g acceleration levels.

Fig. 4 is also used to calculate the apparent crater volume below the original ground surface. Table 4 summarizes the apparent crater volumes, the test conditions, final crater dimensions and the values of the π groups (Schmidt and Holsapple, 1980).

The π groups are dimensionless parameters derived by similar analyses. Schmidt and Holsapple (1980) considered that the crater volume V depends on seven parameters.

$$V = F(G, d, \delta, Y, \rho, a, Q), \quad (1)$$

where G is the acceleration level, d is the depth of burst, δ is the initial density of the explosive, Y is the target material strength parameter, ρ is the initial density of the target material, a is the explosive charge radius, and Q is the energy per unit mass of explosive. The charge mass W can be calculated by

$$W = \frac{4}{3} \pi \delta a^3. \quad (2)$$

Eq. (1) can be rewritten in terms of the five π groups:

$$\pi_1 = F(\pi_2, \pi_3, \pi_4, \pi_5), \quad (3)$$

where

$$\pi_1 = \frac{\rho V}{W}, \quad (4)$$

$$\pi_2 = \frac{G}{Q} \left(\frac{W}{\delta} \right)^{1/3}, \quad (5)$$

$$\pi_3 = d \left(\frac{\delta}{W} \right)^{1/3}, \quad (6)$$

$$\pi_4 = \frac{\rho}{\delta}, \quad (7)$$

$$\pi_5 = \frac{Y}{\delta Q}. \quad (8)$$

If the experiments are conducted in the same sand with the same explosive, π_5 is a constant, π_3 and π_4 can be denoted by d/a and ρ , respectively. In

Table 4 Cratering data summary*

Test No.	ρ (kg/m ³)	W (g)	a (mm)	d (mm)	G ($\times g$)	V (m ³)	π_2	π_V
GE	1.58×10^3	1	5.11	100	1	2.2965×10^{-3}	1.39×10^{-8}	3622.01
CE-1	1.58×10^3	1	5.11	100	40	3.3625×10^{-4}	5.55×10^{-7}	530.34
CE-2	1.58×10^3	1	5.11	100	70	1.0538×10^{-4}	9.71×10^{-7}	165.39
CE-3	1.58×10^3	1	5.11	100	100	8.2990×10^{-5}	1.39×10^{-6}	130.37
CE-4	1.58×10^3	3	7.37	100	40	1.9035×10^{-3}	8.01×10^{-7}	1003.98
CE-5	1.58×10^3	3	7.37	100	70	1.4744×10^{-3}	1.40×10^{-6}	776.23
CE-6	1.58×10^3	3	7.37	100	100	9.5188×10^{-4}	2.00×10^{-6}	503.64
CE-7	1.46×10^3	3	7.37	150	40	6.6374×10^{-4}	8.01×10^{-7}	322.43
CE-8	1.46×10^3	3	7.37	150	70	5.3881×10^{-4}	1.40×10^{-6}	262.88
CE-9	1.46×10^3	3	7.37	150	100	2.7983×10^{-4}	2.00×10^{-6}	137.45

* For all the tests, $\delta=1.786 \times 10^3$ kg/m³ and $Q=5.82 \times 10^6$ m²/s²

Eq. (4), only V is unknown. Thus, π_1 is denoted by π_V . $\pi_V = \pi_1 = \rho V / W$ is the ratio of thrown sand mass to explosive mass, consequently π_V is also called cratering efficiency.

Eq. (5) can be rewritten as

$$\pi_2 = \frac{(G^3 W)^{1/3}}{Q \delta^{1/3}}, \quad (9)$$

where $G^3 W$ is the equivalent charge when explosive fired at a high acceleration level. Here, π_2 is called gravity scaled yield.

Fig. 5 shows the relationship between π_V and π_2 . The results of the ten tests are divided into three groups based on the value of d/a . Each group is fitted to a straight line in the dual-logarithm coordinate system. The relationship between π_V and π_2 is a power regression as

$$\pi_V \pi_2^\alpha = k_V = \text{const}, \quad (10)$$

where α and k_V are constants depending on d/a and ρ . For line No. 1, $d/a=19.6$ and $\rho=1.58 \times 10^3$ kg/m³:

$$\pi_V \pi_2^{0.69} = 0.014. \quad (11)$$

For line No. 2, $d/a=13.6$ and $\rho=1.58 \times 10^3$ kg/m³:

$$\pi_V \pi_2^{0.72} = 0.038. \quad (12)$$

For line No. 3, $d/a=20.3$ and $\rho=1.46 \times 10^3$ kg/m³:

$$\pi_V \pi_2^{0.88} = 0.001. \quad (13)$$

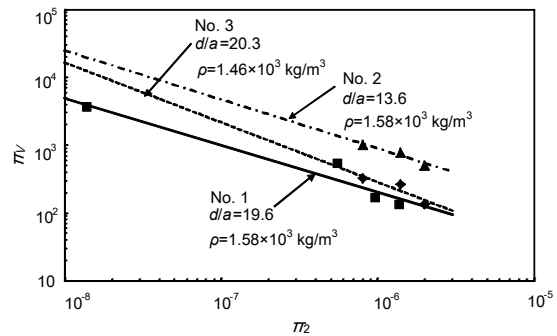


Fig. 5 Cratering efficiency vs. gravity scaled yield

Fig. 6 shows the results of GE, CE-1, CE-2, and CE-3 compared with previous studies (Schmidt *et al.*, 1986; Kutter *et al.*, 1988). The lines obtained by Schmidt *et al.* (1986) and Kutter *et al.* (1988) are close. Correspondingly, the cratering efficiency measured in this study is smaller than theirs. This can be explained by the differences in d/a , ρ and the type of sand. Taking their approximate values of d/a and ρ into account, the results are acceptable.

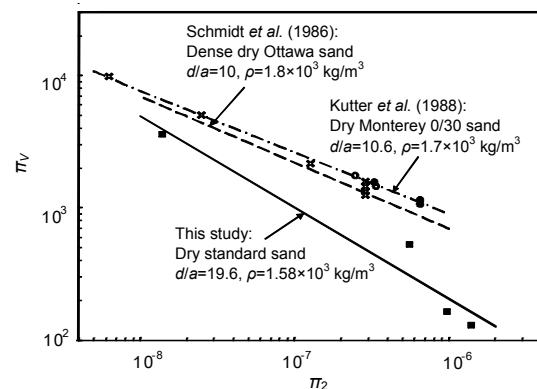


Fig. 6 Comparison of results of this study and previous studies (Schmidt *et al.*, 1986; Kutter *et al.*, 1988)

Schmidt (1977) used π_1 , π_2 and π_3 to analyze charges buried in artist modeling clay. Through the combination of π terms, a scaling law for crater volume in terms of the depth of charge burial was derived:

$$\pi_1 \pi_2^{1/4} = F(\pi_3 \pi_2^{1/6}). \quad (14)$$

Eq. (14) is used to fit the results of ten model tests as shown in Fig. 7.

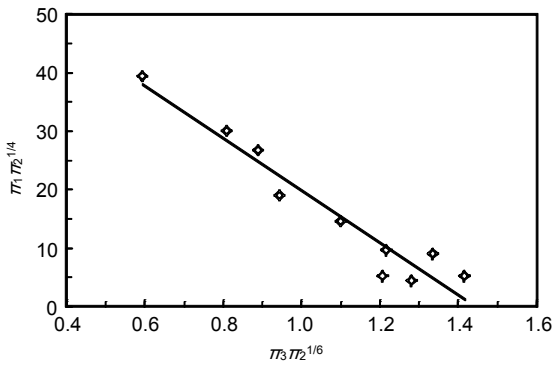


Fig. 7 $\pi_1 \pi_2^{1/4}$ vs. $\pi_3 \pi_2^{1/6}$ in sand

The relationship between $\pi_1 \pi_2^{1/4}$ and $\pi_3 \pi_2^{1/6}$ is approximately linear as follows:

$$\pi_1 \pi_2^{1/4} = -44.669 \pi_3 \pi_2^{1/6} + 64.523, R^2=0.931. \quad (15)$$

Compared with Fig. 5, Fig. 7 is more applicable because of its simple formation and relatively high accuracy. Eq. (15) can be used to predict the crater volume if the attributes of sands and explosives are given.

3.2 Records of accelerations and discussions

The result from accelerometer No. c1 in test CE-6 indicated a pulse with 1250g peak acceleration as shown in Fig. 8. These accelerometers range from -2000g to 2000g, but at the position near explosive, the peak acceleration is invariably out of range, as can be seen from the records of accelerometers No. a1 and No. a2.

Fan et al. (2012) gave the valid peak accelerations recorded by the 11 accelerometers in the ten tests. Fig. 9 shows that the relationship between the proportional peak acceleration and proportional distance bears a power law relationship.

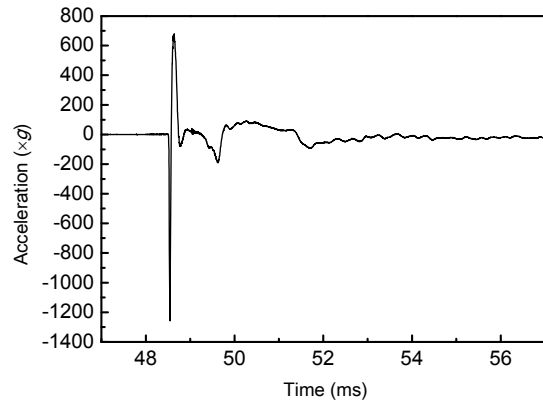


Fig. 8 A typical blast wave (Fan et al., 2012)

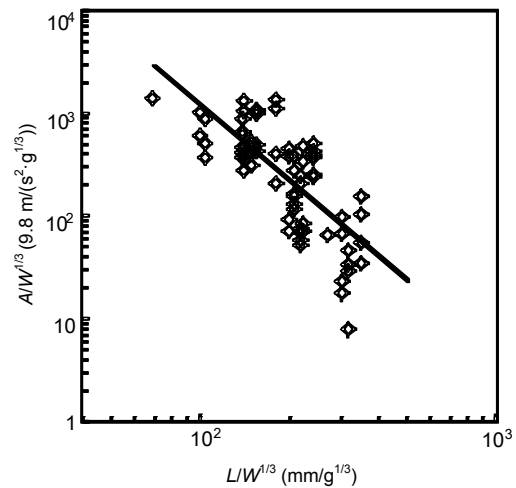


Fig. 9 Curve of peak acceleration vs. proportional distance (Fan et al., 2012)

A is the value of peak acceleration, and L is the distance from accelerometer to explosive

Fig. 10 shows that the proportional peak acceleration is a power increasing function of the gravity scaled yield at a certain distance. The proportional peak acceleration decreases as the distance increases when gravity scaled yield is a constant.

The acceleration wave in Fig. 8 presents important information about arrival time of the blast wave. The point that exhibits sharply turning trench at the very uneven line from the balance station is associated with the arrival time of blast waves. Fig.11 shows the curve of arrival time in each test. The curve of arrival time can be used to calculate the propagation velocity of blast waves traveling through sands. The velocity of blast waves is in the range of 200–714 m/s. The different accelerations result in different

sand densities, and then make the differences in propagation velocities.

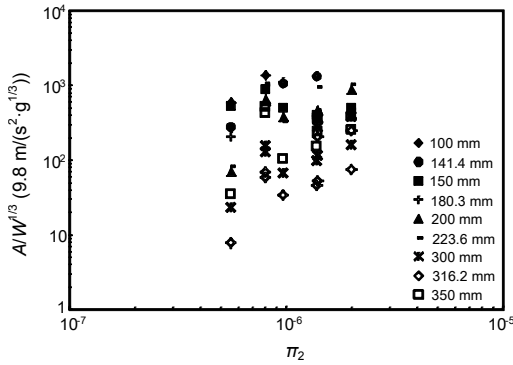


Fig. 10 Curve of peak acceleration vs. gravity scaled yield (Fan et al., 2012)

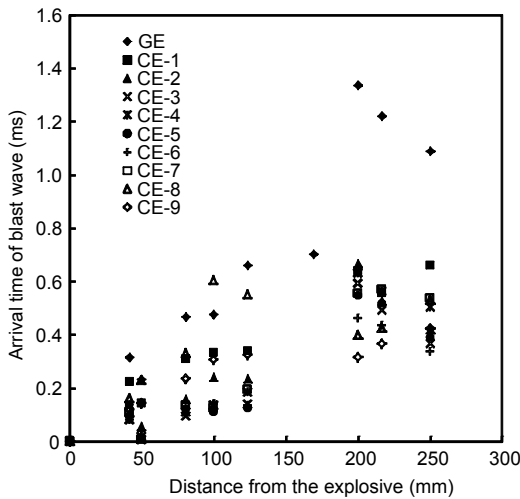


Fig. 11 Curve of arrival time of blast wave

4 Illustrative examples for applications

Two illustrative examples that are concerned with the materials of fuse-plug dams and lunar soils are presented. The gradations of these materials are considered to be close to those used in the experiments described in this study.

4.1 Example 1: Predicting dimensions of the crater in a fuse-plug dam

A fuse-plug dam with the sand density of $1.78 \times 10^3 \text{ kg/m}^3$ is designed to be demolished by blasting. A 50 kg of RDX is assumed to be buried 1 m

depth at the axis of dam. The progress of calculation can be given as

$$\pi_2 = \frac{G}{Q} \left(\frac{W}{\delta} \right)^{1/3} = \frac{9.8}{5.82 \times 10^6} \times \left(\frac{50}{1.786 \times 10^3} \right)^{1/3} = 5.113 \times 10^{-7}, \quad (16)$$

$$\pi_3 = d \left(\frac{\delta}{W} \right)^{1/3} = 1 \times \left(\frac{1.786 \times 10^3}{50} \right)^{1/3} = 3.293. \quad (17)$$

π_1 can be calculated by substituting Eqs. (16) and (17) into Eq. (15):

$$\begin{aligned} \pi_1 &= \frac{-44.669\pi_3\pi_2^{1/6} + 64.523}{\pi_2^{1/4}} \\ &= \frac{-44.669 \times 3.293 \times (5.113 \times 10^{-7})^{1/6} + 64.523}{(5.113 \times 10^{-7})^{1/4}} \\ &= 1920.998. \end{aligned} \quad (18)$$

The volume of the crater can be predicted by substituting Eq. (18) into Eq. (4)

$$V = \frac{\pi_1 W}{\rho} = \frac{1920.998 \times 50}{1.78 \times 10^3} = 53.96 \text{ (m}^3\text{)}. \quad (19)$$

The calculation results show that the explosion will tear a breach with about 53.96 m^3 in the fuse-plug dam. Then a reservoir water level higher than the bottom of the crater may cause a dam breach.

4.2 Example 2: Predicting dimensions of the crater on the moon

In the future, explosives might be used to establish shelter and mine on the moon. The lunar gravity is $0.17g$. The density of lunar regolith is ranged between 1.30×10^3 to $2.29 \times 10^3 \text{ kg/m}^3$ (Carrier et al., 1991). An average value of $1.80 \times 10^3 \text{ kg/m}^3$ is taken into calculation. A 15 kg of RDX is assumed to be buried 0.3 m depth in lunar regolith.

$$\pi_2 = \frac{G}{Q} \left(\frac{W}{\delta} \right)^{1/3} = \frac{0.17 \times 9.8}{5.82 \times 10^6} \times \left(\frac{15}{1.786 \times 10^3} \right)^{1/3} = 5.819 \times 10^{-8}, \quad (20)$$

$$\pi_3 = d \left(\frac{\delta}{W} \right)^{1/3} = 0.3 \times \left(\frac{1.786 \times 10^3}{15} \right)^{1/3} = 1.476. \quad (21)$$

π_1 can be calculated by substituting Eqs. (20) and (21) into Eq. (15):

$$\begin{aligned}\pi_1 &= \frac{-44.669\pi_3\pi_2^{1/6} + 64.523}{\pi_2^{1/4}} \\ &= \frac{-44.669 \times 1.476 \times (5.819 \times 10^{-8})^{1/6} + 64.523}{(5.819 \times 10^{-8})^{1/4}} \quad (22) \\ &= 3890.173.\end{aligned}$$

The volume of the crater can be predicted by substituting Eq. (22) into Eq. (4):

$$V = \frac{\pi_1 W}{\rho} = \frac{3890.173 \times 15}{1.80 \times 10^3} = 32.42 \text{ (m}^3\text{)}. \quad (23)$$

The results show that the explosion will create a crater with the volume of 32.42 m³ in the lunar regolith.

The prediction of the two examples is shown in Fig. 12 by solid circle and hollow circle, respectively.

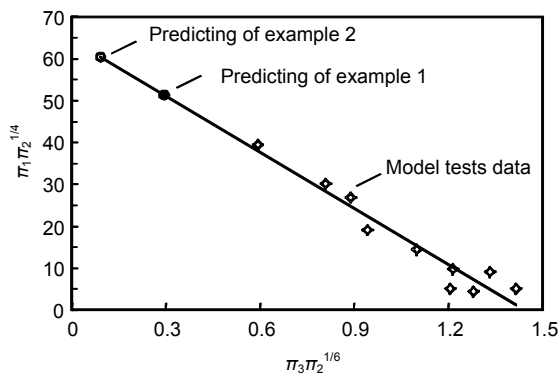


Fig. 12 Predicting of the two examples by Eq. (15)

5 Conclusions

Ten tests of explosion in sand were conducted through the 450 g-t centrifuge at IWHR to investigate the explosion cratering effect and the propagation laws of blast wave. The following conclusions can be drawn.

1. The relationship between cratering efficiency π_V and gravity scaled yield π_2 bears a power law $\pi_V \pi_2^\alpha = k_V = \text{const}$. The values of α and k_V have been measured in the tests, associated with some typical values of d/a and ρ . The cratering efficiency measured

in this study is compared to that obtained by other studies with reasonable agreements. A scaling law that based on the combination of π terms is given as

$$\pi_1 \pi_2^{1/4} = -44.669 \pi_3 \pi_2^{1/6} + 64.523, \quad R^2 = 0.931.$$

The relationship can be conveniently used to predict the cratering effects in sand.

2. The peak acceleration is a power increasing function of the gravity. The proportional peak acceleration is a power regression function of the proportional distance. In dry standard sand the propagation velocity of blast waves is in a range of 200 to 714 m/s in these ten tests.

Some conditions that may affect the results of a centrifuge test, for example, the atmospheric pressure, are not considered in Eq. (15), but the error is possibly appreciable. These limitations will be improved by further studies.

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